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# European Technical Assessment

# ETA-15/0256 of 02/06/2015

English translation prepared by CSTB - Original version in French language

**General Part** 

Technical Assessment Body issuing the ETA and designated according to Article 29 of the Regulation (EU) No 305/2011:

to M24, rebar 8 to 25mm

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08840 Viladecans (Barcelona)

intégrante de cette évaluation

Edificio Australia

DESA

Spain

Plant 1

Chemical Injection System DESA-Chem VSF

dans le béton : M8 à M24, fers à béton 8 à 25mm

Cheville à scellement de type "à injection" pour fixation

Bonded injection type anchor for use in concrete: sizes M8

Nom commercial Trade name

Famille de produit Product family

Titulaire *Manufacturer* 

Usine de fabrication

Cette evaluation contient: This Assessment contains

Manufacturing plants

Base de l'ETE Basis of ETA 22 pages including 19 pages of annexes which form an integral part of this assessment
ETAG 001, Version April 2013, utilisée en tant que EAD ETAG 001, Edition April 2013 used as EAD

22 pages incluant 19 pages d'annexes qui font partie

Cette evaluation remplace: *This Assessment replaces* 

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# Specific part

#### 1 Technical description of the product

The injection system VSF is a bonded anchor (injection type) consisting of a mortar cartridge with DESA-Chem chemical anchoring resin VSF and a steel element. The steel elements are threaded rods made of zinc coated steel, stainless steel, high corrosion resistant stainless steel (HCR), or rebar.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between steel element, injection mortar and concrete. The steel element is intended to be used with embedment depth from 4 diameters to 20 diameters.

The illustration and the description of the product are given in Annexes A.

#### 2 Specification of the intended use

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annexes B.

The provisions made in this European Technical Assessment are based on an assumed working life of the anchor of 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

#### 3 Performance of the product

#### 3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under tension loads for threaded rod Acc. TR029	See Annex C 1
Characteristic resistance under tension loads for rebars Acc. TR029	See Annex C 2
Characteristic resistance under shear loads for threaded rods Acc. TR029	See Annex C 3
Characteristic resistance under shear loads for rebars Acc. TR029	See Annex C 4
Characteristic resistance under tension loads for threaded rods Acc. CEN/TS	See Annex C 5
Characteristic resistance under tension loads for rebars Acc. CEN/TS	See Annex C 6
Characteristic resistance under shear loads for threaded rods Acc. CEN/TS	See Annex C 7
Characteristic resistance under shear loads for rebars Acc. CEN/TS	See Annex C 8
Displacement for threaded rods and rebars	See Annex C 9

#### 3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance	
Reaction to fire	Anchorages satisfy requirements for Class A1	
Resistance to fire	No performance determined (NPD)	

#### 3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances contained in this European Technical Assessment, there may be requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Directive, these requirements need also to be complied with, when and where they apply.

#### 3.4 Safety in use (BWR 4)

For Basic requirement Safety in use the same criteria are valid as for Basic Requirement Mechanical resistance and stability.

- 3.5 Protection against noise (BWR 5) Not relevant.
- 3.6 Energy economy and heat retention (BWR 6) Not relevant.

#### 3.7 General aspects relating to fitness for use

Durability and Serviceability are only ensured if the specifications of intended use according to Annex B1 are kept.

#### 4 Assessment and verification of constancy of performance (AVCP)

According to the Decision 96/582/EC of the European Commission<sup>1</sup>, as amended, the system of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) given in the following table apply.

Product	Intended use	Level or class	System
Metal anchors for use in concrete	For fixing and/or supporting to concrete, structural elements (which contributes to the stability of the works) or heavy units	_	1

#### 5 Technical details necessary for the implementation of the AVCP system

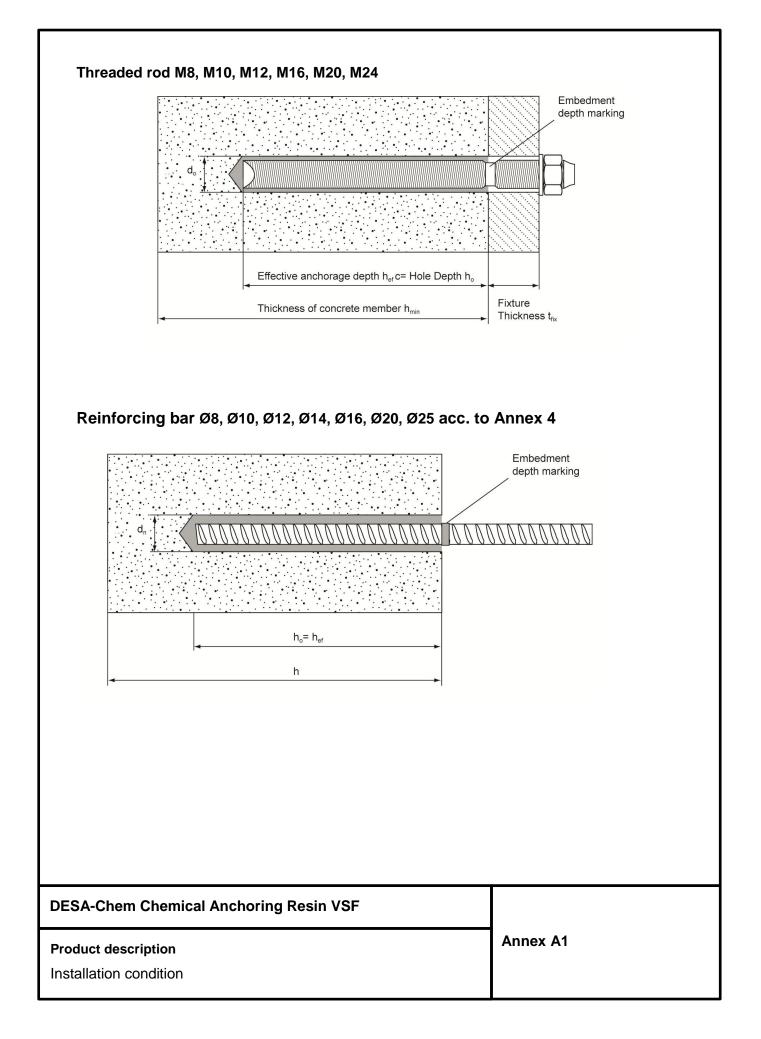
Technical details necessary for the implementation of the Assessment and verification of constancy of performance (AVCP) system are laid down in the control plan deposited at Centre Scientifique et Technique du Bâtiment.

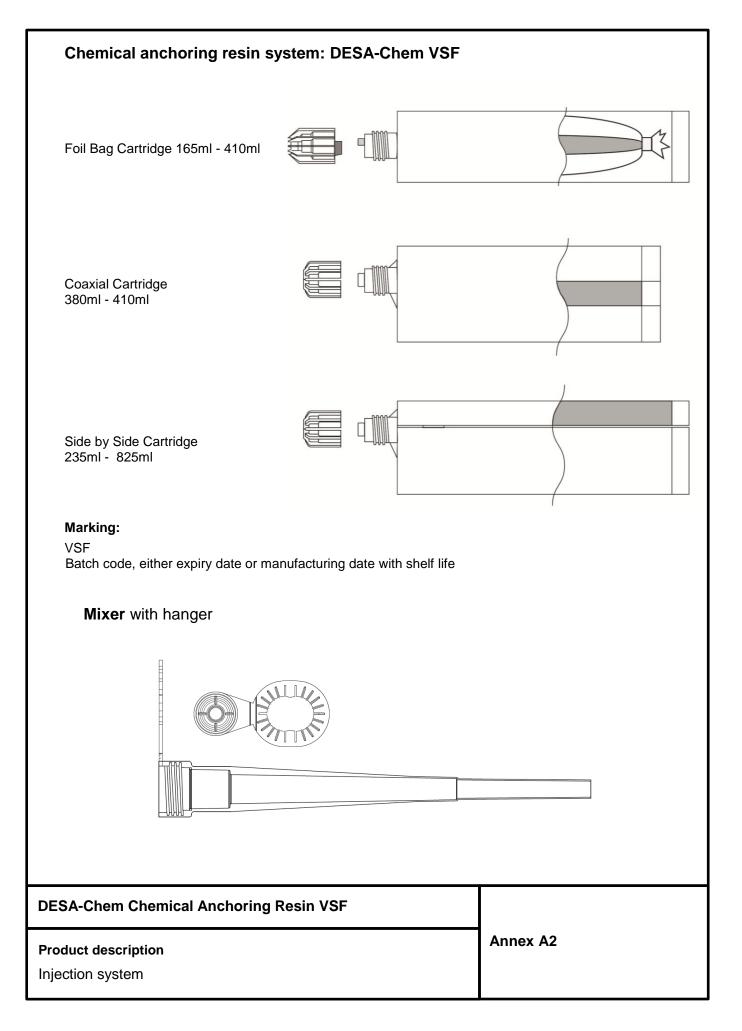
The manufacturer shall, on the basis of a contract, involve a notified body approved in the field of anchors for issuing the certificate of conformity CE based on the control plan.

#### The original French version is signed by

Charles Baloche Technical Director

1





Anchor rod and rebar: Threaded Steel Stud, Nut and Wash Sizes M8, M10, M12, M16, M20, M2				
L hef 				
<b>Rebar</b> Diameter Ø 8mm, Ø 10mm, Ø 12mm, Ø 14mm, Ø 16mi	m, Ø 20mm, Ø 25mm			
000000000000000000000000000000000000000				
DESA-Chem Chemical Anchoring Resin VSF	Annex A3			
Product description Threaded rods and rebars				

### **Table A1: Materials**

Designation	Material			
Threaded rods made of zinc coated steel				
Threaded rod M8 – M24	Strength class 5.8, 8.8, 10.9 EN ISO 898-1, Steel galvanized $\ge$ 5µm EN ISO 4042, Hot dipped galvanized $\ge$ 45 µm EN ISO 10684			
Washer ISO 7089	Steel galvanized EN ISO 4042; hot dipped galvanized EN ISO 10684			
Nut EN ISO 4032	Strength class 8 EN ISO 898-2 Steel galvanized ≥ 5 µm EN ISO 4042 Hot dipped galvanized ≥ 45 µm EN ISO 10684			
Threaded rods made of sta	nless steel			
Threaded rod M8 – M24	For ≤ M24: strength class 70 EN ISO 3506-1; Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088			
Washer ISO 7089	Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088			
Nut EN ISO 4032	Strength class 70 EN ISO 3506-2 Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088			
Threaded rods made of hig				
Threaded rod M8 – M24	For $\leq$ M20: R <sub>m</sub> = 800 N/mm <sup>2</sup> ; R <sub>p0,2</sub> = 640N/mm <sup>2</sup> , For > M20: R <sub>m</sub> = 700 N/mm <sup>2</sup> ; R <sub>p0,2</sub> = 400N/mm <sup>2</sup> , High corrosion resistant steel 1.4529, 1.4565 EN 10088			
Washer ISO 7089	High corrosion resistant steel 1.4529, 1.4565 EN 10088			
Nut EN ISO 4032	Strength class 70 EN ISO 3506-2 High corrosion resistant steel 1.4529, 1.4565 EN 10088			

# Table A2: Properties of reinforcement bars (rebars)

Product form		Bars and de-coiled rods		
Class		В	С	
Characteristic yield streng	th f <sub>yk</sub> or f <sub>0,2k</sub> (MPa)	400 to 600		
Minimum value of $k = (f_t / f_t)$	<sub>y</sub> )k	≥ 1,08	≥ 1,15 < 1,35	
Characteristic strain at ma	ximum force, ε <sub>uk</sub> (%)	≥ 5,0	≥ 7,5	
Bendability		Bend / Rebend test		
Maximum deviation from nominal massNominal bar size (mm) $\leq 8$ (individual bar) (%)		± 6,0 ± 4,5		
Bond: Minimum relative rib area, f <sub>R,min</sub> (determination according to EN 15630)Nominal bar size (mm) 		0,0 0,0		

Height of the rebar rib h<sub>rib</sub>:

The height of the rebar rib  $h_{rib}$  shall fulfil the following requirement: 0,05 \* d ≤  $h_{rib}$  ≤ 0,07 \* d with: d = nominal diameter of the rebar

# Product description

Threaded rods and rebars

# Specifications of intended use

#### Anchorages subject to:

• Static and quasi-static loads

#### **Base materials:**

- Cracked concrete and non-cracked concrete
- Reinforced or unreinforced normal weight concrete of strength classes C 20/25 at least to C50/60 at most according to ENV 206: 2000-12.

#### Temperature Range:

- Ta: 40°C to +40°C (max. short term temperature +40°C and max. long term temperature +24°C)
- Tb: 40°C to +80°C (max. short term temperature +80°C and max. long term temperature +50°C)

#### Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel, high corrosion resistance steel).
- Structures subject to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel, high corrosion resistance steel).
- Structures subject to permanently damp internal condition, with particular aggressive conditions (high corrosion resistance steel).
- Structures subject to external atmospheric exposure including industrial and marine environment if no particular aggressive conditions exist (stainless steel, high corrosion resistance steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

#### Design:

- The anchorages are designed in accordance with the EOTA Technical Report TR 029 "Design of bonded anchors" and CEN/TS 1992-4-5" Design of fastenings for use in concrete" under the responsibility of an engineer experienced in anchorages and concrete work
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings.

#### Installation:

- Dry or wet concrete (category 1).
- Hole drilling by rotary drill mode.
- Overhead installation is not permitted
- Installation in cracked concrete for threaded rods sizes M12 and M16 only
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

DESA-Chem Chemical Anchoring Resin VSF	
Intended Use Specifications	Annex B1

Threaded rod And rebar	Size	Nominal drill bit diameter d <sub>o</sub> (mm)	Steel Brush	Cleaning methods	
		8		Manual cleaning (MAC)	Compressed air cleaning (CAC)
	M8	10	12mm	Yes h <sub>ef</sub> ≤ 80 mm	
Studs	M10	12	14mm	Yes h <sub>ef</sub> ≤ 100mm	
	M12	14	16mm	Yes h <sub>ef</sub> ≤ 120mm	Yes
8	M16	18	20mm	Yes h <sub>ef</sub> ≤ 160mm	
	M20	24	26mm	Yes h <sub>ef</sub> ≤ 200mm	
	M24	28	30mm	Yes h <sub>ef</sub> ≤ 240mm	
	Ø8	12	14mm	Yes h <sub>ef</sub> ≤ 80 mm	
	Ø10	14	16mm	Yes h <sub>ef</sub> ≤ 100mm	
Rebar	Ø12	16	18mm	Yes h <sub>ef</sub> ≤ 120mm	
	Ø14	18	20mm	Yes h <sub>ef</sub> ≤ 140mm	Yes
****	Ø16	20	22mm	Yes h <sub>ef</sub> ≤ 160mm	
	Ø20	25	28mm	Yes h <sub>ef</sub> ≤ 200mm	
	Ø25	32	34mm	Yes h <sub>ef</sub> ≤ 240mm	

#### Manual Cleaning (MAC):

DESA hand pump recommended for blowing out bore holes with diameters  $d_o \le 24$  mm and bore holes depth  $h_o \le 10d$ 

### Compressed air cleaning (CAC):

Recommended air nozzle with an orifice opening of minimum 3,5mm in diameter.

**DESA-Chem Chemical Anchoring Resin VSF** 

Intended Use Cleaning brush Applicator guns Annex B2





	Instructions for use				
Bore hole drilling					
	Drill hole in the substrate to the required embedment depth using the appropriately sized carbide drill bit.				
Bore hole cleaning	Just before setting an anchor, the bore hole must be free of dust and debris.				
a) Manual air cleanir	<b>ig (MAC)</b> for all bore hole diameters $d_0 \le 24$ mm and bore hole depth $h_0 \le 10d$				
X	The DESA manual pump shall be used for blowing out bore holes up to diameters $d_0 \le 24$ mm and embedment depths up to $h_{ef} \le 10d$ .				
	Blow out at least 4 times from the back of the bore hole, using an extension if needed.				
× 4	Brush 4 times with the specified brush size (see Table B1) by inserting the DESA steel brush to the back of the hole (if needed with an extension) in a twisting motion and removing it.				
X 4	Blow out again with manual pump at least 4 times.				
b) Compressed air c	leaning (CAC) for all bore hole diameters $d_o$ and all bore hole depths				
↔ ↔ <sup>6 Bar</sup>	Blow 2 times from the back of the hole (if needed with a nozzle extension) over the whole length with oil-free compressed air (min. 6 b at 6 m <sup>3</sup> /h).				
×2	Brush 2 times with the specified brush size (see Table B1) by inserting the DESA steel brush to the back of the hole (if needed with an extension) in a twisting motion and removing it.				
Bar H	Blow out again with compressed air at least 2 times.				

Annex B3

# DESA-Chem Chemical Anchoring Resin VSF

# Intended Use

Manufacturer Published Installation Instructions

Table B2b: Installation parameters: drilling, hole cleaning and installation					
Instructions for use					
	Remove the threaded cap from the cartridge.				
	Tightly attach the mixing nozzle. Do not modify the mixer in any way. Made sure the mixing element is inside the mixer. Use only the supplied mixer.				
	Insert the cartridge into the DESA dispenser gun.				
Ser.	Discard the initial trigger pulls of adhesive. Depending on the size of the cartridge, an initial amount of adhesive mix must be discarded.				
×	Discard quantities are - 5cm for between 150ml, 300ml & 400ml Foil Pack - 10cm for all other cartridges				
••	Inject the adhesive starting at the back of the hole, slowly withdrawing the mixer with each trigger pull. Fill holes approximately 2/3 full, to ensure that the annular gap between the anchor and the concrete is completely filled with adhesive along the embedment depth.				
	Before use, verify that the threaded rod is dry and free of contaminants. Install the threaded rod to the required embedment depth during the open gel time $t_{ge}$ has elapsed. The working time $t_{gel}$ is given in Table B3.				
The anchor can be loaded after the required curing time t <sub>cure</sub> (see Table B3) The applied torque shall not exceed the values T <sub>max</sub> given in Table B4.					
DESA-Chem Chemical Anche	oring Resin VSF				
<b>ntended Use</b> /lanufacturer Published Installatic	ended Use anufacturer Published Installation Instructions				

# Table B3: Minimum curing time

Minimum base material temperature $C^\circ$		Gel time (working time) t <sub>ge</sub> l In dry/wet concrete	Cure time			
-10°C	$\leq$	T <sub>base material</sub>	<	-5°C	125 min	8 hours
-5°C	$\leq$	T <sub>base material</sub>	<	0°C	80 min	160 min
0°	$\leq$	T <sub>base material</sub>	<	5°C	25 min	90 min
5°C	$\leq$	T <sub>base material</sub>	<	10°C	17 min	70 min
10°C	$\leq$	T <sub>base material</sub>	<	20°C	12 min	65 min
20°C	$\leq$	T <sub>base material</sub>	<	30°C	6 min	60 min
30°C	$\leq$	T <sub>base material</sub>	$\leq$	40°C	3 min	45 min

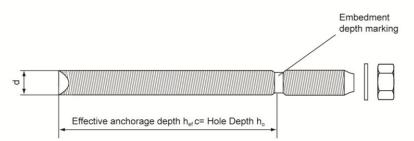
The temperature of the bond material must be  $\ge 20^{\circ}$ C

# **DESA-Chem Chemical Anchoring Resin VSF**

Intended Use Gelling and curing times Annex B4

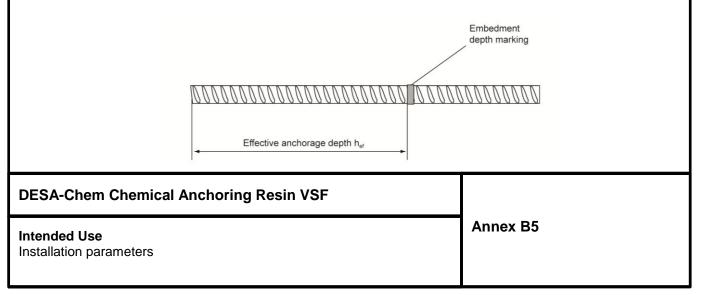
Table B4:	Installation details for anchor rods
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Anchor size			M8	M10	M12	M16	M20	M24
Diameter of anchor rod	d	[mm]	8	10	12	16	20	24
Range of anchorage depth h <sub>ef</sub>	min	[mm]	60	60	70	80	90	100
and bore hole depth ${\sf h}_{\sf o}$	max	[mm]	160	200	240	320	400	480
Nominal anchorage depth	h <sub>ef</sub>	[mm]	80	90	110	125	170	210
Nominal diameter of drill bit	d <sub>o</sub>	[mm]	10	12	14	18	24	28
Diameter of clearance hole in the fixture	d <sub>f</sub>	[mm]	9	12	14	18	22	26
Maximum torque moment	T <sub>max</sub>	[Nm]	10	20	30	60	90	140
Minimum thickness of concrete member	h <sub>min</sub>	[mm]	-	<sub>ef</sub> + 30m : 100mr		h <sub>ef</sub> + 2d <sub>o</sub>		0
Minimum spacing	$S_{min}$	[mm]	40	50	60	80	100	120
Minimum edge distance	$C_{min}$	[mm]	40	50	60	80	100	120



# Table B5 - Installation details for rebars

Rebar Diameter			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Diameter of element	D	[mm]	8	10	12	14	16	20	25
Range of anchorage depth h <sub>ef</sub>	min	[mm]	60	60	70	75	80	90	100
and bore hole depth $h_o$	max	[mm]	160	200	240	280	320	400	500
Nominal diameter of drill bit	d <sub>o</sub>	[mm]	12	14	16	18	20	25	32
Minimum thickness of concrete member	h <sub>min</sub>	[mm]	h <sub>ef</sub> + 30mm ≥ 100mm			h <sub>ef</sub> + 2d <sub>o</sub>			
Minimum spacing	$S_{\text{min}}$	[mm]	40	50	60	70	80	100	125
Minimum edge distance	$\boldsymbol{C}_{min}$	[mm]	40	50	60	70	80	100	125



Steel failure	d rods		M8	M10	M12	M16	M20	M24
Characteristic resistance, class 5.8	N <sub>Rk,s</sub>	[kN]	18	29	42	79	123	177
Characteristic resistance, class 8.8	$N_{Rk,s}$	[kN]	29	46	67	126	196	282
Partial safety factor	γ <sub>Ms,N</sub> 1)	[-]			1	,5	_	-
Characteristic resistance, class 10.9	N <sub>Rk,s</sub>	[kN]	36	58	84	157	245	353
Partial safety factor	γ <sub>Ms,N</sub> 1)	[-]			1	.4		
Characteristic resistance, A4-70	N <sub>Rk,s</sub>	[kN]	26	41	59	110	172	247
Partial safety factor	1) γMs,N	[-]			1,	87	1	1
Characteristic resistance, HCR	N <sub>Rk,s</sub>	[kN]	29	46	67	126	196	247
Partial safety factor	1) γMs,N	[-]			1,5			2,1
Combined Pull-out and Concrete co					[		1	1
Diameter of threaded rod	d	[mm]	8	10	12	16	20	24
Characteristic bond resistance in non-	cracked co	1		_	_	-	_	
Temperature range I <sup>2)</sup> : 40°C/24°C	$\tau_{\text{Rk}}$	[N/mm²]	10.0	9.5	9.0	8.0	7.5	7.0
Temperature range II <sup>2</sup> : 80°C/50°C	$\tau_{\text{Rk}}$	[N/mm²]	9.0	8.0	7.5	7.0	6.5	6.0
ncreasing factor for $\tau_{Rk,p}$	-	C30/37				12		
n non-cracked concrete	$\psi_{c}$		1,23					
		C50/60			1,	30		
Characteristic bond resistance in cract		1	_5)	_5)	25	25	_5)	_5)
Femperature range I <sup>2)</sup> :       40°C/24°C         Femperature range II <sup>2)</sup> :       80°C/50°C	$ au_{Rk}$	[N/mm <sup>2</sup> ]	5)	5)	3.5 3.0	3.5 3.0	5)	5)
Temperature range II : 80°C/50°C	$ au_{Rk}$	[N/mm <sup>2</sup> ] C30/37	- '	- 1		04	- '	- '
ncreasing factor for $\tau_{Rk,p}$	-	C30/37 C40/50				07		
n cracked concrete	$\psi_{c}$	C50/60	1,09					
Splitting failure <sup>2)</sup>		000/00			,	00		
		2)			h/h <sub>ef</sub> ↑			
	h / h	n <sub>ef</sub> <sup>3)</sup> ≥ 2,0	1,0	1,0 h <sub>ef</sub> 4,6 h <sub>ef</sub> - 1,8 h				
dae distance e [mm] for	20 \ h /	h <sub>ef</sub> <sup>3)</sup> > 1,3	46h.					
dge distance c <sub>cr,sp</sub> [mm] for	2,0 > 117	n <sub>et</sub> > 1,3	4,0 Het	- 1,0 11	1,3			
	h /	h <sub>ef</sub> <sup>3)</sup> ≤ 1,3	2,26	h <sub>ef</sub>	+			→ c <sub>cr,sp</sub>
						1,0∙h <sub>ef</sub>	2,26 h <sub>ef</sub>	ci,ap
Spacing	S <sub>cr,sp</sub>	[mm]	()	4)		cr,sp	()	4)
Partial safety factor $\gamma_{Mp} = \gamma_{Mc}$	$_{\rm s} = \gamma_{\rm Msp}^{1)}$	[-]	1,5 <sup>4)</sup>	1,5 <sup>4)</sup>	1,5 <sup>4)</sup>	1,5 <sup>4)</sup>	1,5 <sup>4)</sup>	1,5 <sup>4)</sup>

**Design according to TR 029** Characteristic resistance under tension loads for threaded rods

		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	
N <sub>Rk,s</sub>	[kN]	28	43	62	85	111	173	270	
γ <sub>Ms,N</sub> <sup>3)</sup>	[-]				1,4				
ne failure									
d	[mm]	8	10	12	14	16	20	25	
racked co	ncrete C20/	/25							
$\tau_{\text{Rk}}$	[N/mm²]	7,0	7,5	7,0	7,0	6,5	6,5	6,0	
$\tau_{\text{Rk}}$	[N/mm²]	6.5	6.5	6,0	6,0	6,0	5,5	5,5	
	C30/37	1,12							
$\psi_{c}$	C40/50	1,23							
	C50/60	1,30							
h /	′ h <sub>ef</sub> <sup>5)</sup> ≥ 2,0	1,	0 h <sub>ef</sub>						
2,0 > h /	$h_{ef}^{5)} > 1,3$	4,6 h	<sub>ef</sub> - 1,8 h	1,3	3				
h /	h <sub>ef</sub> <sup>5)</sup> ≤ 1,3	2,2	26 h <sub>ef</sub>			1,0∙h <sub>ef</sub>	2,26 <sup>.</sup> h <sub>ef</sub>	<b>C</b> <sub>cr,sp</sub>	
S <sub>cr,sp</sub>	[mm]				$2 c_{cr,sp}$				
	$\gamma_{MS,N}$ <sup>3)</sup> ne failure d cracked co $\tau_{Rk}$ $\tau_{Rk}$ $\psi_c$ h / 2,0 > h / h /	$\begin{array}{c} \gamma_{\text{Ms,N}} ^{3)} & [-] \\ \hline \textbf{ne failure} \\ \hline \textbf{d} & [mm] \\ \hline \textbf{cracked concrete C20/} \\ \hline \tau_{\text{Rk}} & [\text{N/mm}^2] \\ \hline \tau_{\text{Rk}} & [\text{N/mm}^2] \\ \hline \tau_{\text{Rk}} & [\text{N/mm}^2] \\ \hline \psi_c & \hline \textbf{C30/37} \\ \psi_c & \hline \textbf{C40/50} \\ \hline \textbf{C50/60} \\ \hline \textbf{h / h_{ef}}^{5)} \geq 2,0 \\ \hline 2,0 > \text{h / h_{ef}}^{5)} \geq 1,3 \\ \hline \textbf{h / h_{ef}}^{5)} \leq 1,3 \end{array}$	$\begin{array}{c c c c c c } & [kN] & 28 \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & &$	$\begin{array}{c c c c c c } & N_{Rk,s} & [kN] & 28 & 43 \\ \hline & & & & & \\ \hline & & & & & \\ \hline & & & &$	$\begin{array}{c c c c c c c c } \hline N_{Rk,s} & [kN] & 28 & 43 & 62 \\ \hline \gamma_{Ms,N}^{3)} & [-] & & & \\ \hline \textbf{ne failure} & & & \\ \hline \textbf{me failure} & & & \\ \hline \textbf{d} & [mm] & 8 & 10 & 12 \\ \hline \textbf{me failure} & & & \\ \hline \textbf{d} & [mm] & 8 & 10 & 12 \\ \hline \textbf{cracked concrete C20/25} & & & \\ \hline \tau_{Rk} & [N/mm^2] & 7,0 & 7,5 & 7,0 \\ \hline \tau_{Rk} & [N/mm^2] & 6.5 & 6.5 & 6,0 \\ \hline \tau_{Rk} & [N/mm^2] & 6.5 & 6.5 & 6,0 \\ \hline \textbf{cso/37} & & & \\ \hline \psi_{c} & \hline \textbf{C30/37} & & & \\ \hline \psi_{c} & \hline \textbf{C30/37} & & & \\ \hline \psi_{c} & \hline \textbf{C30/37} & & & \\ \hline \psi_{c} & \hline \textbf{C30/37} & & & \\ \hline \textbf{h} & / \ h_{ef}^{5)} \geq 2,0 & \textbf{1,0 h_{ef}} & & \\ \hline \textbf{h} & / \ h_{ef}^{5)} \geq 1,3 & \textbf{4,6 h_{ef}} - \textbf{1,8 h} & & \\ \hline \textbf{h} & / \ h_{ef}^{5)} \leq 1,3 & \textbf{2,26 h_{ef}} \end{array}$	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	

<sup>1)</sup> The characteristic tension resistance  $N_{Rk,s}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.1).

<sup>2)</sup> The partial safety factor  $\gamma_{Ms,N}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (3.3a).

<sup>3)</sup> In absence of national regulations

<sup>4)</sup> Explanation see Annex B1

 $\frac{5}{10}$  h concrete member thickness, h<sub>ef</sub> effective anchorage depth

<sup>6)</sup> The partial safety factor  $\gamma_2 = 1,2$  is included.

<b>DESA-Chem</b>	Chemical	Anchoring	<b>Resin VSF</b>
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#### Design according to TR 029

Characteristic resistance under tension loads for rebars

DESA-Chem VSF with threaded r	ods		M 8	M 10	M 12	M 16	M 20	M 24	
Steel failure without lever arm				•					
Characteristic resistance, class 5.8	$V_{Rk,s}$	[kN]	9	15	21	39	61	88	
Characteristic resistance, class 8.8	$V_{\text{Rk,s}}$	[kN]	15	23	34	63	98	141	
Characteristic resistance, class 10.9	V <sub>Rk,s</sub>	[kN]	18	29	42	79	123	156	
Characteristic resistance, A4-70	$V_{\text{Rk,s}}$	[kN]	13	20	30	55.0	86	124	
Characteristic resistance, HCR	$V_{Rk,s}$	[kN]	15	23	34	62.8	98	124	
Steel failure with lever arm									
Characteristic resistance, class 5.8	$M^0_{Rk,s}$	[Nm]	19	37	66	167	326	561	
Characteristic resistance, class 8.8	$M^0_{Rk,s}$	[Nm]	30.0	60	105	266	519	898	
Characteristic resistance, class 10.9	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	38	75	131	333	649	893	
Characteristic resistance, A4-70	$M^0_{Rk,s}$	[Nm]	26	26 53 92 233 454					
Characteristic resistance, HCR	$M^0_{Rk,s}$	[Nm]	30 60 105 266 519					786	
Partial safety factor steel failure									
grade 5.8 or 8.8	1) γ <sub>Ms,V</sub>	[-]			1,	25			
grade 10.9	γ <sub>Ms,V</sub> 1)	[-]			1,	50			
A4-70	γMs,V <sup>1</sup>	[-]			1,	56			
HCR	γ <sub>Ms,V</sub> 1)	[-]			1,25			1,75	
Concrete pryout failure									
Factor in equation (5.7) of Technical Report TR029 for the design of bonded anchors	k	[-]	2,0						
Partial safety factor	1) γ <sub>Mcp</sub>	[-]			1,	5 <sup>2)</sup>			
Concrete edge failure <sup>3)</sup>									
Partial safety factor	γ <sub>Mc</sub> <sup>1)</sup>	[-]			1,5	5 <sup>2)</sup>			

1) In absence of national regulations

2)

The partial safety factor  $\gamma_{2}$ = 1.0 is included Concrete edge failure see chapter 5.2.3.4 of Technical Report TR029 3)

### Design according to TR 029

Characteristic resistance under shear loads for threaded rods

DESA-Chem VSF with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure without lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 488 <sup>1)</sup>	$V_{Rk,s}$	[kN]	14	22	31	42	55	86	135
Partial safety factor for rebar BSt 500 S acc. to DIN 488 <sup>2)</sup>	γMs,∨ <sup>3)</sup>	[-]				1,5			
Steel failure with lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 488 <sup>4)</sup>	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]	33	65	112	178	265	518	1012
Partial safety factor for rebar BSt 500 S acc. to DIN 488 <sup>3)</sup>	γ <sub>Ms,V</sub> <sup>3)</sup>	[-]				1,5			
Concrete pryout failure									
Factor in equation (5.7) of Technical Report TR029 for the design of bonded anchors	k	[-]				2,0			
Partial safety factor	γ <sub>Mcp</sub> <sup>3)</sup>	[-]	1,5 <sup>5)</sup>						
Concrete edge failure <sup>6)</sup>									
Partial safety factor	γ <sub>Mc</sub> <sup>3)</sup>	[-]				1,5 <sup>5)</sup>			

<sup>1)</sup> The characteristic shear resistance  $V_{Rk,s}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6).

<sup>2)</sup> The partial safety factor  $\gamma_{Ms,N}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation 3.3b or 3.3c.

<sup>3)</sup> In absence of national regulations

<sup>4)</sup> The characteristic bending resistance  $M^0_{Rk,s}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6b).

<sup>5)</sup> The partial safety factor  $\gamma_2 = 1,0$  is included.

<sup>6)</sup> Concrete edge failure see chapter 5.2.3.4 of Technical Report TR029

#### **DESA-Chem Chemical Anchoring Resin VSF**

#### **Design according to TR 029** Characteristic resistance under shear loads for rebars

DESA-Chem VSF with threaded re	oas		M 8	M 10	M 12	M 16	M 20	M 2	
Steel failure				1	1	1	n	r	
Characteristic resistance, class 5.8	N <sub>Rk,s</sub>	[kN]	18	29	42	79	123	177	
Characteristic resistance, class 8.8	N <sub>Rk,s</sub>	[kN]	29	46	67	126	196	282	
Partial safety factor	1) γMs,N	[-]		1	1.				
Characteristic resistance, class 10.9	N <sub>Rk,s</sub>		36	58	84	157	245	353	
Partial safety factor	1) γms,N			1	1.4	40			
Characteristic resistance "A4 70"	N <sub>Rk,s</sub>	[kN]	26	41	59	110	172	247	
Partial safety factor	1) γMs,N	[-]			1.8	37			
Characteristic resistance "HCR"	N <sub>Rk,s</sub>	[kN]	29	46	67	126	196	247	
Partial safety factor	γMs,N <sup>1)</sup>	[-]	] 1.5						
Combined Pull-out and Concrete cone	failure								
Diameter of threaded rod	d	[mm]	8	10	12	16	20	24	
Characteristic bond resistance in non-crac	cked concrete C2	20/25							
Temperature range I <sup>2)</sup> : 40°C/24°C	$ au_{Rk,uncr}$	[N/mm²]	10.0	9.5	9.0	8.0	7.5	7.0	
Temperature range II <sup>2)</sup> : 80°C/50°C	$ au_{Rk,uncr}$	[N/mm²]	9.0	8.0	7.5	7.0	6.5	6.0	
		C30/37		•	1,'	12		•	
ncreasing factor for $\tau_{Rk,p}$	Ψc	C40/50			1,2				
n non-cracked concrete	10	C50/60			1,3	30			
Characteristic bond resistance in cracked	concrete C20/25				,				
Temperature range I <sup>2)</sup> : 40°C/24°C	$ au_{Rk,cr}$	[N/mm²]	_5)	<b>-</b> <sup>5)</sup>	3.5	3.5	_ <sup>5)</sup>	_ <sup>5)</sup>	
Temperature range II <sup>2)</sup> : 80°C/50°C	$ au_{Rk,cr}$	[N/mm²]	_5)	- <sup>5)</sup>	3.0	3.0	_ <sup>5)</sup>	_ <sup>5)</sup>	
		C30/37			1,0	)4			
ncreasing factor for $\tau_{Rk}$	ψc C40/				1,0				
n cracked concrete	C50/60				1,0	)9			
Factor according to CEN/TS 1992-4-5	k8 non cracked concr	ete [-]	10.1						
Section 6.2.2	k <sub>8 cracked concrete</sub>	[-]			7.	2			
Concrete cone failure									
Factor according to CEN/TS 1992-4-5	k <sub>ucr</sub>	[-]			10				
Section 6.2.3	k <sub>cr</sub>	[-]	7.2						
Edge distance	C <sub>cr,N</sub>	[-]	1,5 h <sub>ef</sub>						
Axial distance	S <sub>cr,N</sub>	[-]			3,0	n <sub>ef</sub>			
Splitting failure									
	h/ł	$n_{\rm ef}^{3)} \ge 2.0$	1,0 ŀ	امل	n/h <sub>ef</sub> ↑				
		01 , -	,-		2,0 -				
Talana dia tanàna amin'ny fisiana dia mampikana	20× h/	h <sub>ef</sub> <sup>3)</sup> > 1,3	166.	4.0.1					
Edge distance c <sub>cr,sp</sub> [mm] for	2,0 > 11 /	n <sub>ef</sub> '> 1,3	4,0 N <sub>ef</sub> -	1,011	1,3 -				
		2)							
	h /	h <sub>ef</sub> <sup>3)</sup> ≤ 1,3	2,26	h <sub>ef</sub>	+	10.6	2,26 ⋅h <sub>ef</sub>	→ c <sub>cr,s</sub>	
						1,0 ∙h <sub>ef</sub>	∠,∠o <sup>.</sup> n <sub>ef</sub>		
	S <sub>cr,sp</sub> [mm]		2 . C <sub>cr,sp</sub>						
Spacing	$S_{cr,sp}$ $\gamma_{Mp} = \gamma_{Mc} = \gamma_{Ms}$		1,5 <sup>4)</sup>						

# **DESA-Chem Chemical Anchoring Resin VSF**

### Design according to CEN/TS 1992-4

Characteristic resistance under tension loads for threaded rods

DESA-Chem VSF with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure rebar									
Characteristic resistance for rebar BSt 500 S acc. to DIN 488	1) N <sub>Rk,s</sub>	[kN]	28	43	62	85	111	173	270
Partial safety factor for rebar BSt 500 S acc. to DIN 488	2) γMs,N <sup>3)</sup>	[-]				1,4			
Combined Pull-out and Concrete	cone failure	;							
Diameter of rebar	d	[mm]	8	10	12	14	16	20	25
Characteristic bond resistance in nor	n-cracked co	ncrete C20/	/25						
Temperature range I <sup>4)</sup> : 40°C/24°C	C $\tau_{Rk}$	[N/mm²]	7,0	7,5	7,0	7,0	6,5	6,5	6,0
Temperature range II <sup>4</sup> : 80°C/50°C	C τ <sub>Rk</sub>	[N/mm²]	6.5	6.5	6,0	6,0	6,0	5,5	5,5
		C30/37				1,12			
ncreasing factor for $ au_{Rk,p}$ n non-cracked concrete		C40/50				1,23			
		C50/60				1,30			
Factor according to CEN/TS 1992-4-5 Section 6.2.2	k <sub>8 non cracked c</sub>	concrete [-]				10.1			
Concrete cone failure									
Factor according to CEN/TS 1992- Section 6.2.2	-4-5 ku	ucr [-]				10.1			
Splitting failure									
	h /	/ h <sub>ef</sub> <sup>5)</sup> ≥ 2,0	1,	0 h <sub>ef</sub>	h/ł	P <sub>ef</sub>			
			-		2,0	ייייד ס			
Edge distance $c_{cr,sp}$ [mm] for	2,0 > h /	′ h <sub>ef</sub> <sup>5)</sup> > 1,3	4,6 h	<sub>≆f</sub> - 1,8 h	1,3	3			
	h /	′ h <sub>ef</sub> <sup>5)</sup> ≤ 1,3	2,2	26 h <sub>ef</sub>			1,0 ⋅ h <sub>ef</sub>	2,26 <sup>.</sup> h <sub>ef</sub>	C <sub>cr,sp</sub>
Spacing	S <sub>cr,sp</sub>	[mm]				2 c <sub>cr,sp</sub>			
Partial safety factor $\gamma_{Mp} = \gamma_{1}$	$\gamma_{\rm Mc} = \gamma_{\rm Msp}^{3)}$	[-]	1,8 <sup>6)</sup>	1,8 <sup>6)</sup>	1,8 <sup>6)</sup>	1,8 <sup>6)</sup>	1,8 <sup>6)</sup>	1,8 <sup>6)</sup>	1,8 <sup>6</sup>

<sup>1)</sup> The characteristic tension resistance  $N_{Rk,s}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.1).

<sup>2)</sup> The partial safety factor  $\gamma_{Ms,N}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (3.3a).

<sup>3)</sup> In absence of national regulations

<sup>4)</sup> Explanation see Annex B1

 $\frac{5}{2}$  h concrete member thickness, h<sub>ef</sub> effective anchorage depth

<sup>6)</sup> The partial safety factor  $\gamma_2 = 1,2$  is included.

# **DESA-Chem Chemical Anchoring Resin VSF**

### Design according to CEN/TS 1992-4

Characteristic resistance under tension loads for rebars

DESA-Chem VSF with threaded r	ods		M 8	M 10	M 12	M 16	M 20	M 24
Steel failure without lever arm								
Characteristic resistance, class 5.8	$V_{\text{Rk},\text{s}}$	[kN]	9	15	21	39	61	88
Characteristic resistance, class 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141
Characteristic resistance, class 10.9	$V_{Rk,s}$	[kN]	18	29	42	79	123	156
Characteristic resistance, A4-70	$V_{\text{Rk},\text{s}}$	[kN]	13	20	30	55.0	86	124
Characteristic resistance, HCR	$V_{\text{Rk},\text{s}}$	[kN]	15	23	34	62.8	98	124
Steel failure with lever arm								
Characteristic resistance, class 5.8	$M^0_{Rk,s}$	[Nm]	19	37	66	167	326	561
Characteristic resistance, class 8.8	$M^0_{\rm Rk,s}$	[Nm]	30.0	60	105	266	519	898
Characteristic resistance, class 10.9	$M^0_{\rm Rk,s}$	[Nm]	38	75	131	333	649	893
Characteristic resistance, A4-70	$M^0_{Rk,s}$	[Nm]	26	53	92	233	454	625
Characteristic resistance, HCR	$M^0_{Rk,s}$	[Nm]	30	30 60 105 266 519				
Partial safety factor steel failure								
grade 5.8 or 8.8	γ <sub>Ms,V</sub> 1)	[-]			1.	,25		
grade 10.9	γ <sub>Ms,V</sub> 1)	[-]			1,	,50		
A4-70	γMs,V <sup>1)</sup>	[-]			1.	,56		
HCR	γMs,V <sup>1)</sup>	[-]			1,25			1,75
Concrete pryout failure								
Factor according to CEN/TS 1992-4-5 Section 4.3.3	k <sub>3</sub>	[-]			2	2,0		
Partial safety factor	1) γ <sub>Mcp</sub>	[-]			1,	,5 <sup>2)</sup>		
Concrete edge failure								
Partial safety factor	γ <sub>Mc</sub> <sup>1)</sup>	[-]			1,	,5 <sup>2)</sup>		

1)

In absence of national regulations The partial safety factor  $\gamma_{2}\text{=}$  1.0 is included 2)

# **DESA-Chem Chemical Anchoring Resin VSF**

### Design according to CEN/TS 1992-4

Characteristic resistance under shear loads for threaded rods

DESA-Chem VSF with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure without lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 488 <sup>1)</sup>	$V_{Rk,s}$	[kN]	14	22	31	42	55	86	135
Partial safety factor for rebar BSt 500 S acc. to DIN 488 <sup>2)</sup>	γ <sub>Ms,V</sub> <sup>3)</sup>	[-]				1,5			
Steel failure with lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 488 <sup>1)</sup>	$M^0_{\rm Rk,s}$	[Nm]	33	65	112	178	265	518	1012
Partial safety factor for rebar BSt 500 S acc. to DIN 488 <sup>2)</sup>	γ <sub>Ms,V</sub> <sup>3)</sup>	[-]	1,5						
Concrete pryout failure									
Factor according to CEN/TS 1992-4-5 Section 4.3.3	k <sub>3</sub>	[-]				2,0			
Partial safety factor	3) γ <sub>Mcp</sub>	[-]	1,5 <sup>5)</sup>						
Concrete edge failure									
Partial safety factor	γ <sub>Mc</sub> <sup>3)</sup>	[-]				1,5 <sup>5)</sup>			

1) The characteristic shear resistance  $V_{Rk,s}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6).

2) The partial safety factor  $\gamma_{Ms,N}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation 3.3b or 3.3c.

<sup>3)</sup> In absence of national regulations

4) The characteristic bending resistance  $M^0_{Rk,s}$  for rebars that do not fulfil the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6b). 5)

The partial safety factor  $\gamma_2 = 1,0$  is included.

**DESA-Chem Chemical Anchoring Resin VSF** 

Design according to CEN/TS 1992-4 Characteristic resistance under shear loads for rebars

<b>DESA-Chem VSF wit</b>	M8	M10	M12	M16	6 M:	20	M24		
Non cracked concrete	temperature ra	nge I <sup>7)</sup> : 40°C / 2	4°C						
Displacement	δ <sub>N0</sub>	[mm/(N/mm <sup>2</sup> )]	0,03	0,03	0,04	0,05	5 0,0	06	0,07
Displacement	$\delta_{N_\infty}$	[mm/(N/mm <sup>2</sup> )]	0,07	0,09	0,10	0,13	<b>0</b> ,	17	0,20
Non cracked concrete	temperature ra	nge II <sup>7)</sup> : 80°C / 5	0°C					<u> </u>	
Displacement	δ <sub>N0</sub>	[mm/(N/mm <sup>2</sup> )]	0,04	0,04	0,05	0,07	<b>'</b> 0,0	08	0,10
Displacement	$\delta_{N_\infty}$	[mm/(N/mm²)]	0,10	0,13	0,15	0,19	0,2	23	0,28
Cracked concrete temp	perature range	I <sup>7)</sup> : 40°C / 24°C							
Displacement	$\delta_{N0}$	[mm/(N/mm <sup>2</sup> )]	-	-	0,12	0,09	) -		-
Displacement	$\delta_{N\infty}$ [mm/(N/mm <sup>2</sup> )]		-	-	0,64	0,55		-	-
Cracked concrete temp	perature range	II <sup>7)</sup> : 80°C / 50°C							
Displacement	$\delta_{N0}$	[mm/(N/mm <sup>2</sup> )]	-	-	0,17	0,13	3 -		-
Displacement	$\delta_{N_\infty}$	[mm/(N/mm²)]	-	-	0,90	0,78	3 -		-
DESA-Chem VSF wit	h rebar		Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø2
Temperature range I <sup>9)</sup> :	40°C / 24°C								
Displacement	δ <sub>N0</sub>	[mm/(N/mm²)]	0,03	0,03	0,04	0,04	0,05	0,06	0,07
Displacement	$\delta_{N_\infty}$	[mm/(N/mm²)]	0,07	0,09	0,10	0,12	0,13	0,17	0,20
Temperature range II <sup>9)</sup>	: 80°C / 50°C								
Disulas sus sut	δησ	[mm/(N/mm²)]	0,04	0,04	0,05	0,06	0,07	0,08	0,10
Displacement	UNU	[	,						-

 $^{1)}$  Calculation of displacement under service load:  $\tau_{\text{Sd}}$  design value of bond stress

Displacement under short term loading =  $\delta_{N0} \cdot \tau_{Sd}/1,4$ 

Displacement under long term loading =  $\delta_{N\infty} \cdot \tau_{Sd}/1.4$ 

# Displacement under shear load <sup>2)</sup>

DESA-Chem VSF with threaded rods			M8	M10	M12	M16	M20	M24
Displacement	$\delta_{V0}$	[mm/kN]	0,06	0,06	0,05	0,04	0,04	0,03
Displacement	$\delta_{V_\infty}$	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05

DESA-Chem VSF with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Displacement	$\delta_{V0}$	[mm/kN]	0,06	0,05	0,05	0,04	0,04	0,04	0,03
Displacement	$\delta_{V\!\infty}$	[mm/kN]	0,09	0,08	0,07	0,06	0,06	0,05	0,05

 $^{2)}$  Calculation of displacement under service load: V<sub>Sd</sub> design value of shear load. Displacement under short term loading =  $\delta_{N0} \cdot V_{Sd}/1,4$  Displacement under long term loading =  $\delta_{V\infty} \cdot V_{Sd}/1,4$ 

DESA-Chem Chemical Anchoring Resin VSF	
<b>Design</b> Anchor displacements	Annex C9